

Analisi numerica di problemi di ingegneria strutturale

Margherita Porcelli

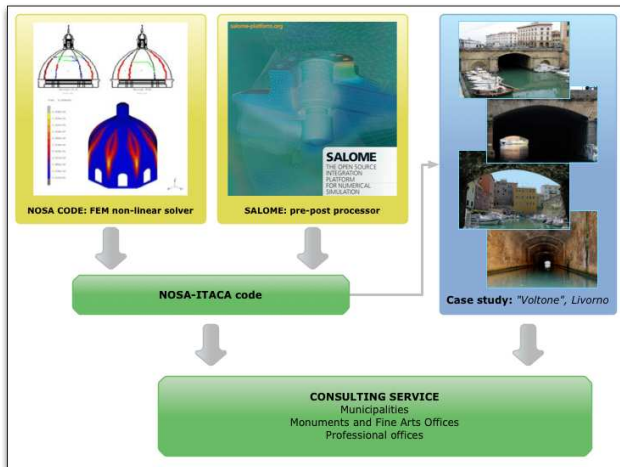


Seminari di Analisi Numerica
Marzo 11, 2013 - Pisa

The NOSA-ITACA project (2011-2013, Cultural Heritage)

- ▶ Partners: ISTI -CNR Research Area of Pisa
Department of Constructions and Restoration - UNIFI
- ▶ **Application**: to support the activities of safeguarding and strengthening historic masonry constructions.
- ▶ **Numerical analysis**: methods for large constrained eigenvalue problems.
- ▶ Funded by PAR FAS Regione Toscana

Tools for the modelling and assessment of the structural behaviour of ancient masonry constructions: the NOSA-ITACA code



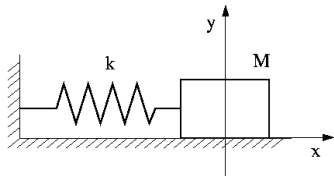
The modal analysis

- ▶ The modal analysis is the study of the dynamic properties of structures under vibrational excitation.

Free vibration equilibrium equations with damping neglected

$$M\ddot{U} + KU = 0$$

- $U \in \mathbb{R}^n$ is the FE displacement vector (time dependent).
- $K, M \in \mathbb{R}^{n \times n}$ are the stiffness and mass matrix of the FE assemblage.
- n = total number of degrees of freedom (DOF) of the system.



- ▶ The solution of the equilibrium equation can be postulated to be of the form

$$U = u \sin(\omega(t - t_0)),$$

- ▶ $u \in \mathbb{R}^n$, t is the time variable, t_0 a time constant,
- ▶ ω a constant that represents the **frequency of vibration** (rad/sec) of the vector u .

Find the $s \ll n$ smallest eigenpairs of the generalized eigenproblem (GEP)

$$K u = \omega^2 M u$$

- ▶ The solution of the (GEP) allows to find the various periods at which a structure naturally resonates.
- ▶ The number of frequencies of interest is mainly related to the geometry of the structure and its mass distribution.



Constraints: $C u = 0, C \in \mathbb{R}^{m \times n}, m \ll n$

Fixed (single-point) constraints

$$u_i = 0, i \in I_F$$

where u_i is the displacement of a single DOF (e.g. imposition Dirichlet boundary conditions).

Master-Slave (multi-points) constraints or tying relations

There exists a subset $I_S \subset \{1, \dots, n\}$ such that

$$u_s = \sum_{m \in I_{M_s}} c_{sm} u_m, \quad s \in I_S, m \in I_{M_s} \subset \{1, \dots, n\} \setminus I_S$$

u_s is the **slave** DOF whereas u_m are the **master** DOFs.

- ▶ These constraints are crucial, e.g., in modeling the contact interaction between masonry and reinforcement.



The constrained eigenvalue problem

$$K u = \lambda M u \quad \text{subject to } C u = 0$$

$K, M \in \mathbb{R}^{n \times n}$ pos. semidef. sparse and symmetric, $C \in \mathbb{R}^{m \times n}$ full row rank and K is pos. def. on the $\text{Null}(C)$.

Unconstrained reformulations

- ▶ **Condensed** problem: let $Z \in \mathbb{R}^{n \times (n-m)}$ be an orthonormal matrix whose columns span $\text{null}(C)$,

$$(Z^T K Z) v = \lambda (Z^T M Z) v, \quad v = Z^T u \in \mathbb{R}^{(n-m)}$$

[Hitziger, Mackens, Voss 1995]

- ▶ **Augmented** problem:

$$\begin{bmatrix} K & C^T \\ C & 0 \end{bmatrix} \begin{bmatrix} u \\ y \end{bmatrix} = \lambda \begin{bmatrix} M & 0 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} u \\ y \end{bmatrix}, y \in \mathbb{R}^m.$$

[Baker, Lehoucq 2007]



Forming a basis for $null(C)$

Lemma

Let $C \in \mathbb{R}^{m \times n}$ a full row rank matrix and let $C = GH$ a **rank-retaining factorization**, i.e. let $G \in \mathbb{R}^{m \times m}$, $H \in \mathbb{R}^{m \times n}$ with $rank(G) = rank(H) = m$.

Moreover, let H be partitioned as

$$H = [H_m \ H_{n-m}]$$

$$H_m \in \mathbb{R}^{m \times m} \text{ non singular and } H_{n-m} \in \mathbb{R}^{m \times (n-m)}.$$

Then, the columns of the matrix $Z \in \mathbb{R}^{n \times (n-m)}$ given by

$$Z = \begin{bmatrix} -H_m^{-1} H_{n-m} \\ I_{n-m} \end{bmatrix}$$

form a basis for $null(C)$.



Results for Master-Slave constraints

Let the columns of C be ordered so that $C = [C_{I_F} C_{I_S} C_{I_M} C_{I_U}]$, then

$$C = GH, \text{ with } G = I_m, H = C$$

is a rank-retaining factorization and

$$H_m = [C_{I_F} C_{I_S}] \text{ and } H_{n-m} = [C_{I_M} 0].$$

Then

$$Z = \begin{bmatrix} -[C_{I_M} & 0] \\ I_{n-m} \end{bmatrix}$$

- ▶ If K, M are sparse \Rightarrow the projected matrices $Z^T K Z, Z^T M Z$ are sparse ($m \ll n$).



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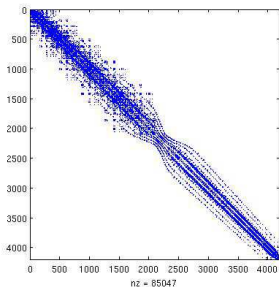
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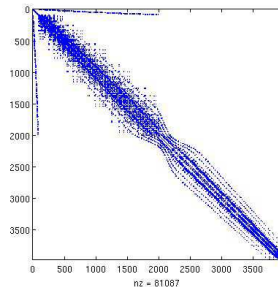
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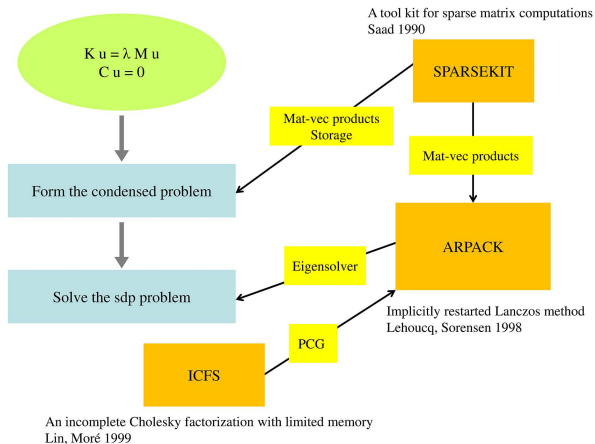
Sparsity pattern of the (left) and of its projection (right) of a walls-roof structure



Stiffness matrix K



Projected stiffness matrix $Z^T K Z$



- ▶ Performed CPU time competitive with MARC (MSC software).
- ▶ In progress: comparison with DACG [Gambolati, Bergamaschi, Pini 1997] and JD [Sleijpen, Van der Vost, Bai 1999]



Case studies

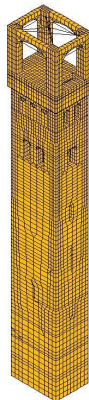
- ▶ Torre delle Ore (Lucca): Fixed constraints
- ▶ S. Francesco (Lucca): Fixed and Master-Slave constraints of the form

$$u_s = u_m, \quad s \in I_S, m \in I_M$$

	n	$ I_F $	$ I_S $	$ I_M $
TdO	46164	320	0	0
SF	61881	2662	54	54

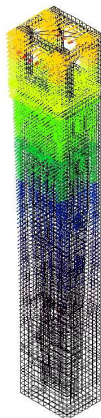
	$m = I_F + I_S $	$N = n - m$	bandwidth
TdO	320	45844	1239
SF	2716	59165	11769

Torre delle Ore



Torre delle Ore: $\nu = 0.67$

Inc: 0:1
Time: 0.000e+00
Freq: 6.729e-01

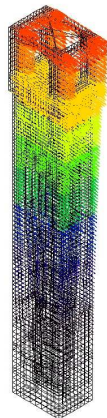


Displacement



Torre delle Ore: $\nu = 0.88$

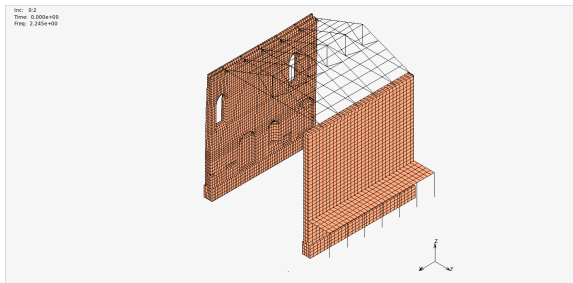
Inc: 0.2
Time: 0.000e+00
Freq: 8.881e-01

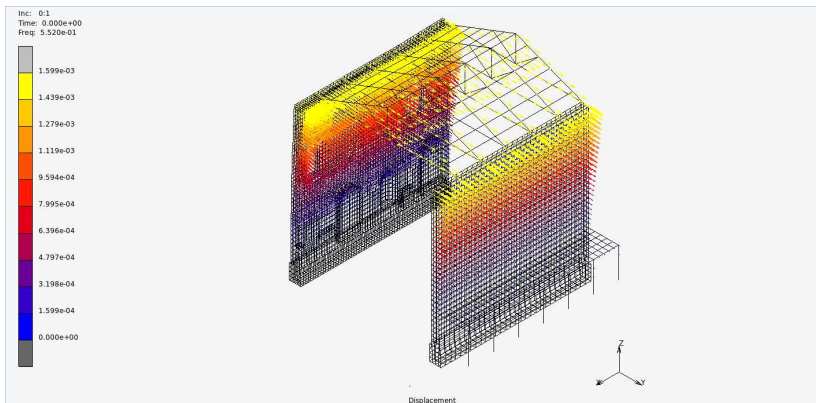


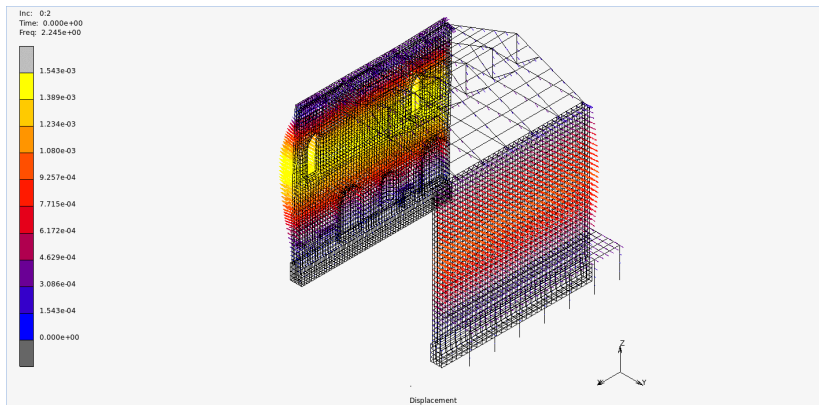
Displacement



S. Francesco



S. Francesco: $\nu_1 = 0.552$ 

S. Francesco: $\nu = 2.245$ 

Thanks for your attention